# STORMWATER MANAGEMENT REPORT

for

### AMENDED PRELIMINARY & FINAL MAJOR SITE PLAN

Prepared for:

# LINDEN DEVELOPMENT, LLC. PROPOSED FREDDY'S FROZEN CUSTARD & STEAKBURGERS WITH DRIVE-THRU

Block 469, Lot 38.05 Pleasant Street and Edgar Road (N.J.S.H. Route 1 & 9) Union County City of Linden, New Jersey

Prepared by:



N.J. Certificate of Authorization 24GA28161700

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BENJ File No. JS200709

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July 2020

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# **Appendices**

#### A. Pre- vs. Post-Development Hydrographs

- Drainage Diagram
- 2-Year Storm Event
- 10-Year Storm Event
- 25-Year Storm Event
- 100-Year Storm Event

#### B. Design Calculations

• Pipe Sizing Calculations

#### C. Maps

- Tax Map
- Aerial Map
- Soil Map
- Depth to Water Table Map
- State Planning Area Map
- USGS
- HUC-14 Map
- FEMA Firm Map
- Drainage Area Maps
  - Proposed Drainage Areas Map
  - Proposed Inlet Areas Map

#### Memorandum

The purpose of this study is to analyze the stormwater drainage conditions that will occur as a result of the proposed Freddy's Frozen Custard & Steakburgers (Freddy's). The Freddy's will include one restaurant building with drive-thru, parking areas, stormwater conveyance and utility systems, landscaping, lighting and other ancillary improvements. The subject site is situated along the intersection of Pleasant Street and Route 1 & 9, within the City of Linden, in the County of Union, New Jersey. The subject site is part of an overall development more specifically defined as Block 469, Lots 38.05 and 38.06 and has a total area of 42.54 Acres.

This report is prepared as an amendment to the Stormwater Management Report for the Proposed Retail Development dated March 2014 and revised through January 2018 previously approved as part of Planning Board Application No. SP-1034-14 and the Stormwater Management Report Amended Preliminary & Final Major Site Plan for Phase 2 and Amended Preliminary Major Site Plan for Phase 3 dated March 2019 previously approved as part of Planning Board Application No. SP-1034-14 and No. SP-1099-19 achieved compliance with applicable stormwater regulations by reducing the impervious cover from 100% to 90% which is the maximum impervious coverage allowed in the PCD Zone. Lot 38.05, involved in the current application, is part of Proposed Drainage Areas #1 and #2 in the referenced applications. Please refer to Proposed Drainage Areas Map in the appendix for more information.

#### Proposed Drainage Area #1:

Proposed Drainage Area 1 consists of 1,192,380 SF of impervious surfaces and 90,272 SF of grassed and landscaped areas. This area is tributary to Pleasant Street right-of-way and ultimately is conveyed to the Linden Avenue drainage system, similar to EDA-1. Runoff from this area will sheet-flow to and be collected in the proposed inlets within the parking areas of the site and conveyed into the existing stormwater system within Pleasant Street. A time of concentration of 13.4 minutes was utilized for this area per the maximum time of concentration calculated in the pipe calculations included in the previously approved Planning Board Application No. SP-1099-19. The additional grassed and landscaped areas provide for natural and non-structural means of water quality as well as an overall reduction in flow to the existing drainage system.

#### Proposed Drainage Area #2:

Proposed Drainage Area 2 consists of 475,563 SF of impervious surfaces and 95,055 SF of grass and landscaped areas. This area is tributary to the Route 1 & 9 right-of-way. A portion of the runoff from this area will sheet-flow directly to the Route 1 & 9 right-of-way, and the other portion will sheet-flow to, and be collected in the proposed inlets within the parking areas of the site and conveyed into the existing stormwater system within Route 1 & 9. A time of concentration of 13.4 minutes was utilized for this area per the maximum time of concentration calculated in the pipe calculations included in the previously approved Planning Board Application No. SP-1099-19. The additional grassed and landscaped areas provide for natural and non-structural means of water quality as well as an overall reduction in flow to the existing drainage system.

#### Conclusion:

The current application proposes 1,667,943 SF of impervious coverage which equates to 90.00% of Block 468, Lots 38.05& 38.06, therefore, the proposed improvements conform with the conditions approved under Applications No. SP-1034-14 and No. SP-1099-19. The stormwater design calculations have been updated accordingly and are attached along with stormwater conveyance calculations, the Proposed Drainage Areas Map and the Proposed Inlet Areas Map in the Appendices.

The following tables summarize the total peak discharge rates for existing and proposed conditions:

Design Storm	(A) Pre-Development Runoff Rate (cfs)	(B) Post-Development Runoff Rate (cfs)	Reduction in Peak Rate (cfs)
2-year	121.28	113.89	7.39
10-year	186.23	178.29	7.49
25-year	231.71	223.74	7.79

Pre vs Post Runoff Rates (EDA VS PDA)
(For Reference-Previously Approved Under App No. SP 1034-14)

1				
	100-year	314.16	306.48	7.68

#### Pre vs Post Runoff Rates (EDA VS PDA)

(For Reference-Previously Approved Under App No. SP 1099-19)

Design Storm	(A) Pre-Development Runoff Rate (cfs)	(B) Post-Development Runoff Rate (cfs)	Reduction in Peak Rate (cfs)
2-year	121.28	103.72	17.56
10-year	186.23	162.39	23.84
25-year	231.71	203.82	27.89
100-year	314.16	279.21	34.95

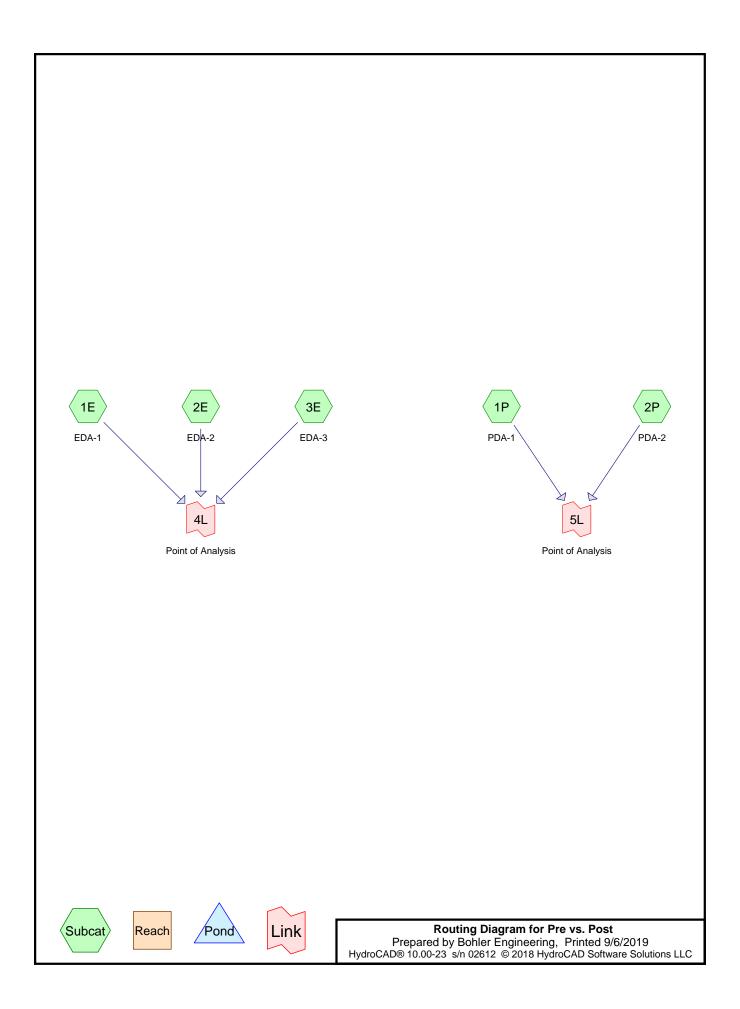
Pre vs Post Runoff Rates (EDA VS PDA) (Revised per this Amended Preliminary & Final Major Site Plan - September 2019)

Design Storm	(A) Pre-Development Runoff Rate (cfs)	(B) Post-Development Runoff Rate (cfs)	Reduction in Peak Rate (cfs)
2-year	121.28	103.72	17.56
10-year	186.23	162.39	23.84
25-year	231.71	203.82	27.89
100-year	314.16	279.22	34.94

### A. PRE- vs. POST-DEVELOPMENT HYDROGRAPHS

- Drainage Diagram
- 2-Year Storm Event
- 10-Year Storm Event
- ◆ 25-Year Storm Event
- 100-Year Storm Event

# DRAINAGE DIAGRAM



2-YEAR STORM EVENT

# Area Listing (all nodes)

# Area (sq-ft) CN (subcatchment-numbers) 3,521,213 98 Impervious (1E, 1P, 2E, 2P, 3E) 185,327 74 Pervious (1P, 2P) 3,706,540 97 TOTAL AREA

### Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
0	HSG A	
0	HSG B	
0	HSG C	
0	HSG D	
3,706,540	Other	1E, 1P, 2E, 2P, 3E
3,706,540		TOTAL AREA

# Pre vs. Post

Prepared by Bohler Engineering HydroCAD® 10.00-23 s/n 02612 © 2018 HydroCAD Software Solutions LLC

Printed 9/6/2019

Ground Covers (all nodes)									
HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover	Subcatchment Numbers		
0	0	0	0	3,521,213	3,521,213	Impervious	1E		
							, 1P		
							, 2E		
							, 2P		
							, 3E		
0	0	0	0	185,327	185,327	Pervious	1P		
							, 2P		
0	0	0	0	3,706,540	3,706,540	TOTAL AREA			

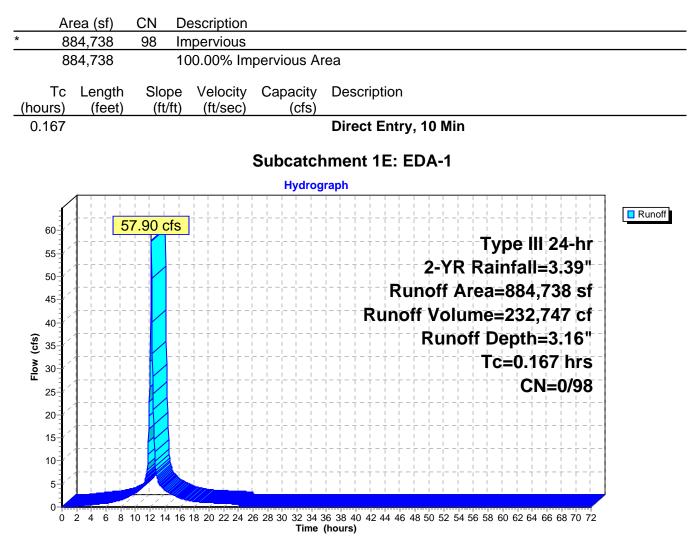
Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1E: EDA-1	Runoff Area=884,738 sf 100.00% Impervious Runoff Depth=3.16" Tc=0.167 hrs CN=0/98 Runoff=57.90 cfs 232,747 cf
Subcatchment 1P: PDA-1	Runoff Area=1,282,652 sf 92.96% Impervious Runoff Depth=3.02" Tc=0.223 hrs CN=74/98 Runoff=73.17 cfs 322,439 cf
Subcatchment 2E: EDA-2	Runoff Area=710,936 sf 100.00% Impervious Runoff Depth=3.16" Tc=0.167 hrs CN=0/98 Runoff=46.52 cfs 187,025 cf
Subcatchment 2P: PDA-2	Runoff Area=570,618 sf 83.34% Impervious Runoff Depth=2.82" Tc=0.223 hrs CN=74/98 Runoff=30.55 cfs 134,331 cf
Subcatchment 3E: EDA-3	Runoff Area=257,596 sf 100.00% Impervious Runoff Depth=3.16" Tc=0.167 hrs CN=0/98 Runoff=16.86 cfs 67,765 cf
Link 4L: Point of Analysis	Inflow=121.28 cfs 487,537 cf
	Primary=121.28 cfs 487,537 cf
Link 5L: Point of Analysis	Inflow=103.72 cfs 456,770 cf
	Primary=103.72 cfs 456,770 cf
Total Dunoff Area - 2 706	E40 of Bunoff Volume - 044 207 of Average Bunoff Donth - 2.06

Total Runoff Area = 3,706,540 sf Runoff Volume = 944,307 cf Average Runoff Depth = 3.06" 5.00% Pervious = 185,327 sf 95.00% Impervious = 3,521,213 sf

#### Summary for Subcatchment 1E: EDA-1

Runoff = 57.90 cfs @ 12.14 hrs, Volume= 232,747 cf, Depth= 3.16"



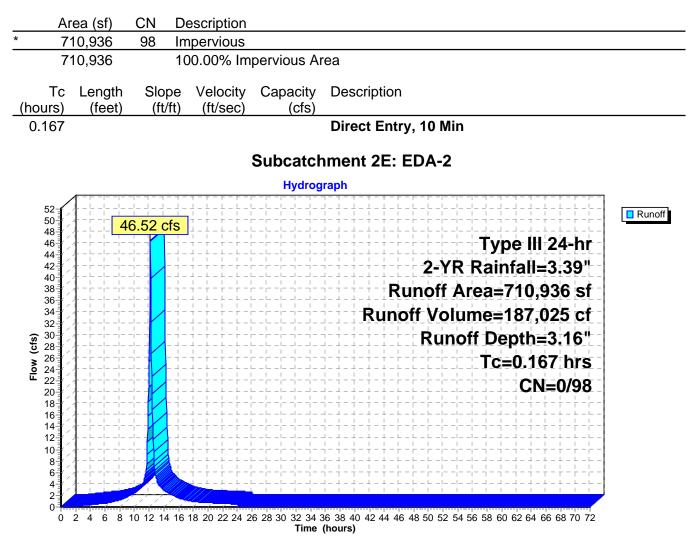
#### Summary for Subcatchment 1P: PDA-1

Runoff = 73.17 cfs @ 12.18 hrs, Volume= 322,439 cf, Depth= 3.02"

	Area (s	f) CN	Description	<u>ו</u>		
*	1,192,38		Impervious	6		
	<u>90,27</u> 1,282,65		Pervious Weighted	Average		
	90,27		7.04% Per			
	1,192,38	0	92.96% Im	pervious Are	ea	
	Tc Len	gth Slo	pe Velocity	/ Capacity	Description	
(houi			t/ft) (ft/sec			
0.2	23				Direct Entry, 13.4	
				Subcate	hment 1P: PDA-1	
				Hydrog		
8		73.17	cfs +-+-+			Runoff
7: 7(	「二」「二二二				Type III 24-hr	
6					2-YR Rainfall=3.39"	
6	0				Runoff Area=1,282,652 sf	
5			+ - + - + - + - + - + - + - + - +		Runoff Volume=322,439 cf	
5 ( <b>s</b> 4					Runoff Depth=3.02"	
4: 4: 4: ۲: 4: (cfs)	1/10/07				Tc=0.223 hrs	
Ы Ы Э					CN=74/98 -	
3	0				······································	
2	5				$\begin{array}{cccccccccccccccccccccccccccccccccccc$	
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						,
	0 2 4 6	6 8 10 12 1	4 16 18 20 22 24		36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 (hours)	

#### Summary for Subcatchment 2E: EDA-2

Runoff = 46.52 cfs @ 12.14 hrs, Volume= 187,025 cf, Depth= 3.16"



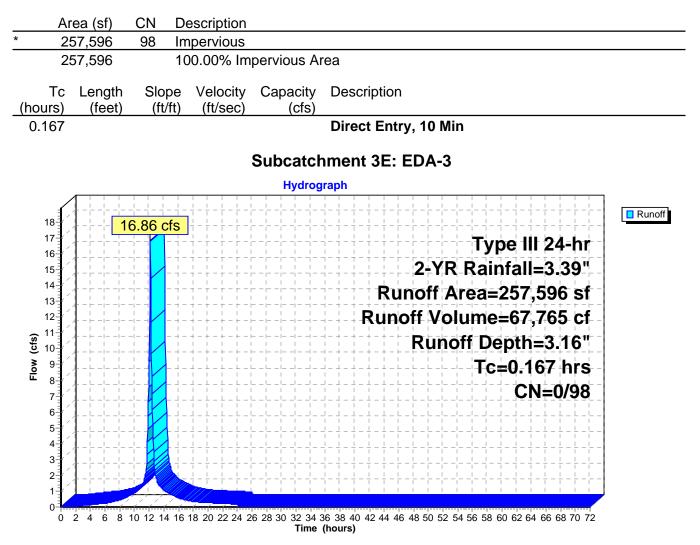
#### Summary for Subcatchment 2P: PDA-2

Runoff = 30.55 cfs @ 12.18 hrs, Volume= 134,331 cf, Depth= 2.82"

	Area (sf)	CN D	escription			
*	475,563		npervious			
*	<u>95,055</u> 570,618		ervious	(0.F0.G0		
	95,055	-	eighted Av 6.66% Per	vious Area		
	475,563			ervious Are	a	
	Tc Length	Slope	Velocity	Capacity	Description	
<u>(hou</u> 0.2		(ft/ft)	(ft/sec)	(cfs)	Direct Entry, 13.4	
0.2	.20				Direct Litty, 13.4	
				Subcatch	nment 2P: PDA-2	
				Hydrog	ıraph	
3	34-1		+-+-+-+-	           	<pre></pre>	Runoff
3	32	80.55 cfs		 	· · · · · · · · · · · · · · · · · · ·	
	30		+-+-+-+-		Type III 24-hr	
					2-YR Rainfall=3.39"	
	26		$\frac{1}{1} = \frac{1}{1} = \frac{1}{1} = \frac{1}{1} = \frac{1}{1} = \frac{1}{1}$	<u> </u> - <u> </u> - <u> </u> - <u> </u> - <u> </u> - <u> </u>	Runoff Area=570,618 sf	
					Runoff Volume=134,331 cf	
(s) <sup>2</sup>	20				Runoff Depth=2.82"	
>	8		$\frac{1}{1} = \frac{1}{1} = \frac{1}{1} = \frac{1}{1} = \frac{1}{1}$	<mark> </mark> <mark> </mark> <mark> </mark>	Tc=0.223 hrs	
Elo	16-1 				CN=74/98	
	<sup></sup>		+-+-+-+	 	<b></b>	
1	10					
	8					
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	0					)
	0 2 4 6 8	10 12 14 16	18 20 22 24 2		36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 (hours)	

#### Summary for Subcatchment 3E: EDA-3

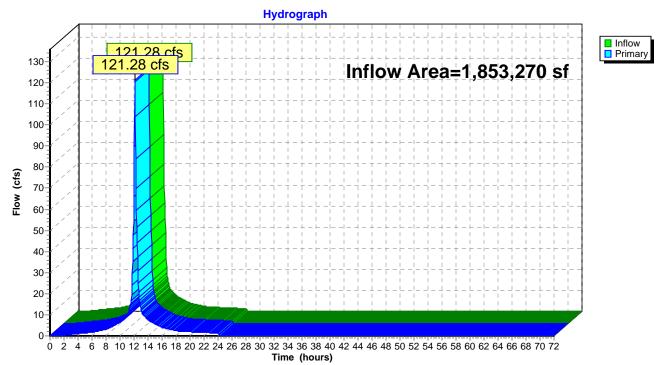
Runoff = 16.86 cfs @ 12.14 hrs, Volume= 67,765 cf, Depth= 3.16"



#### Summary for Link 4L: Point of Analysis

Inflow Are	a =	1,853,270 sf,100.00% Impervious, Inflow Depth = 3.16" for 2-YR event	
Inflow	=	121.28 cfs @ 12.14 hrs, Volume= 487,537 cf	
Primary	=	121.28 cfs @ 12.14 hrs, Volume= 487,537 cf, Atten= 0%, Lag= 0.0 m	nin

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs



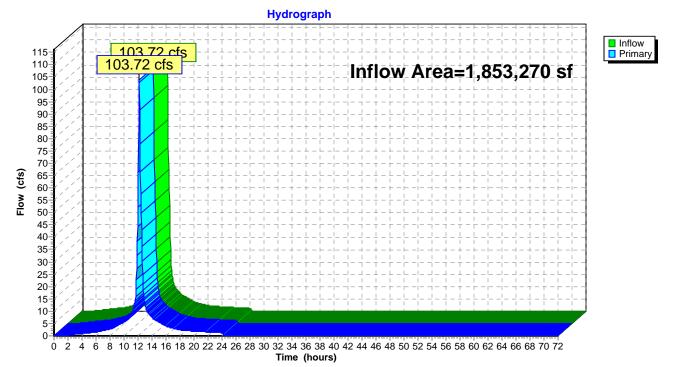
#### Link 4L: Point of Analysis

#### Summary for Link 5L: Point of Analysis

Inflow Are	a =	1,853,270 sf, 90.00% Impervious, Inflow Depth = 2.96" for 2-YR event	
Inflow	=	103.72 cfs @ 12.18 hrs, Volume= 456,770 cf	
Primary	=	103.72 cfs @ 12.18 hrs, Volume= 456,770 cf, Atten= 0%, Lag= 0.0 mir	٦

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

#### Link 5L: Point of Analysis



**10-YEAR STORM EVENT** 

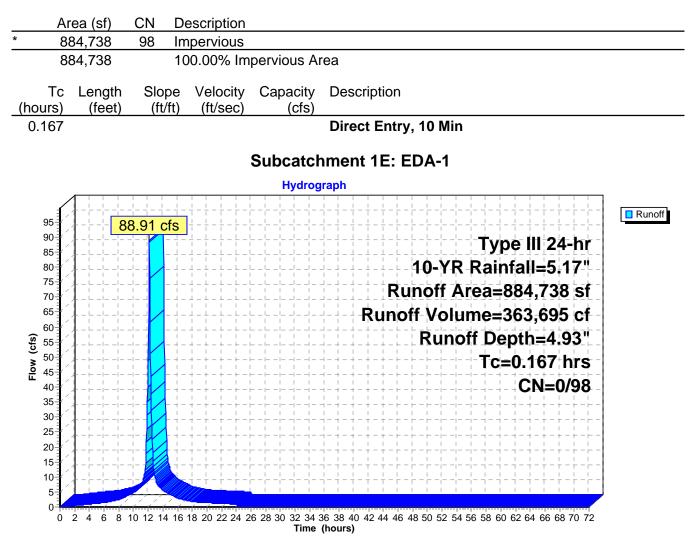
Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1E: EDA-1	Runoff Area=884,738 sf 100.00% Impervious Runoff Depth=4.93" Tc=0.167 hrs CN=0/98 Runoff=88.91 cfs 363,695 cf
Subcatchment1P: PDA-1	Runoff Area=1,282,652 sf 92.96% Impervious Runoff Depth=4.76" Tc=0.223 hrs CN=74/98 Runoff=113.89 cfs 508,970 cf
Subcatchment 2E: EDA-2	Runoff Area=710,936 sf 100.00% Impervious Runoff Depth=4.93" Tc=0.167 hrs CN=0/98 Runoff=71.44 cfs 292,249 cf
Subcatchment 2P: PDA-2	Runoff Area=570,618 sf 83.34% Impervious Runoff Depth=4.53" Tc=0.223 hrs CN=74/98 Runoff=48.50 cfs 215,300 cf
Subcatchment3E: EDA-3	Runoff Area=257,596 sf 100.00% Impervious Runoff Depth=4.93" Tc=0.167 hrs CN=0/98 Runoff=25.89 cfs 105,892 cf
Link 4L: Point of Analysis	Inflow=186.23 cfs 761,835 cf Primary=186.23 cfs 761,835 cf
Link 5L: Point of Analysis	Inflow=162.39 cfs 724,270 cf Primary=162.39 cfs 724,270 cf

Total Runoff Area = 3,706,540 sf Runoff Volume = 1,486,105 cf Average Runoff Depth = 4.81" 5.00% Pervious = 185,327 sf 95.00% Impervious = 3,521,213 sf

#### Summary for Subcatchment 1E: EDA-1

Runoff = 88.91 cfs @ 12.14 hrs, Volume= 363,695 cf, Depth= 4.93"



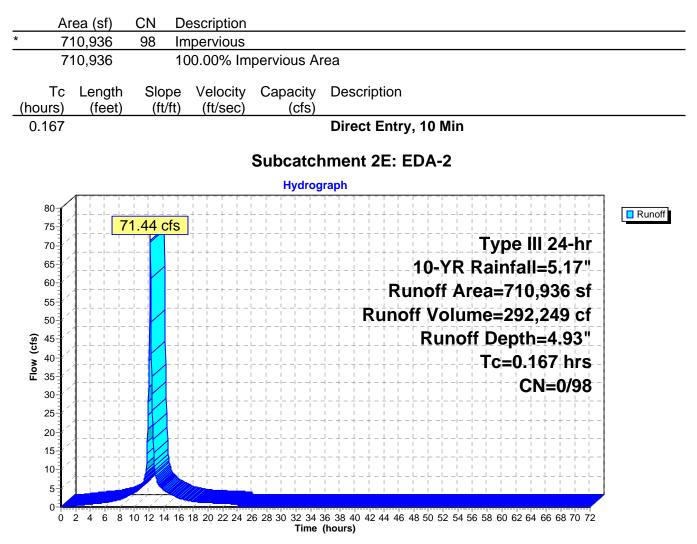
#### Summary for Subcatchment 1P: PDA-1

Runoff = 113.89 cfs @ 12.18 hrs, Volume= 508,970 cf, Depth= 4.76"

	rea (sf)	CN D	escription			
	92,380 90,272		npervious ervious			
1,2	90,272 282,652 90,272 92,380	96 W	/eighted Av .04% Pervi		a	
Tc (hours)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
0.223	(1001)	(1011)	(17300)	(00)	Direct Entry, 13.4	
				Subcatch	nment 1P: PDA-1	
				Hydrog		
Flow (cts) 125 1100 (cts) 90 (cts) 90 850 77 65 60 550 45 40 40 77 100 70 70 70 70 70 70 70 70 70 70 70 70 7		<b>13.89</b> cfs 			Type III 24-hr 10-YR Rainfall=5.17" Runoff Area=1,282,652 sf Runoff Volume=508,970 cf Runoff Depth=4.76" Tc=0.223 hrs CN=74/98	Runoff

#### Summary for Subcatchment 2E: EDA-2

Runoff = 71.44 cfs @ 12.14 hrs, Volume= 292,249 cf, Depth= 4.93"



#### Summary for Subcatchment 2P: PDA-2

Runoff = 48.50 cfs @ 12.18 hrs, Volume= 215,300 cf, Depth= 4.53"

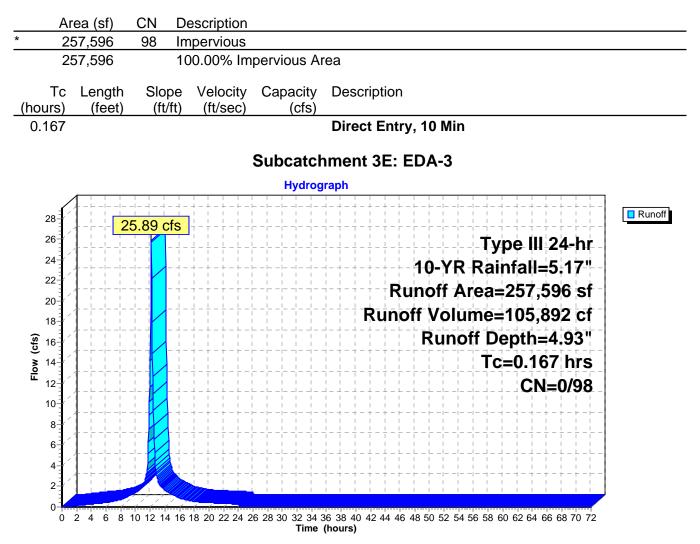
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 10-YR Rainfall=5.17"

	Ar	ea (sf)	CN	Description			
*		'5,563	98	Impervious			
*		5,055	74	Pervious			
	9	70,618 95,055	94	Weighted Av 16.66% Per	vious Area		
	47	5,563		83.34% Imp	ervious Are	ea	
	Тс	Length	Slop		Capacity	Description	
<u>(hou)</u> 0.2		(feet)	(ft/1	ft) (ft/sec)	(cfs)	Direct Entry, 13.4	
0.2	20						
					Subcatch	nment 2P: PDA-2	
					Hydrog	jraph	
	1						Runoff
5	50 <u>-</u>	4	8.50 cl	S		Type III 24-hr	
4	15 					10-YR Rainfall=5.17"	
4	10					Runoff Area=570,618 sf	
3	5 					Runoff Volume=215,300 cf	
(cfs)	80 <del>-</del>				           	Runoff Depth=4.53"	
Flow (cfs)	25-1				i iii I I I I I I I I I I	Tc=0.223 hrs	
_	20						
1	- 5-			$ \begin{array}{cccccccccccccccccccccccccccccccccccc$			
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1	0-1						
	5-						
	0	(inition for the					

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

#### Summary for Subcatchment 3E: EDA-3

Runoff = 25.89 cfs @ 12.14 hrs, Volume= 105,892 cf, Depth= 4.93"

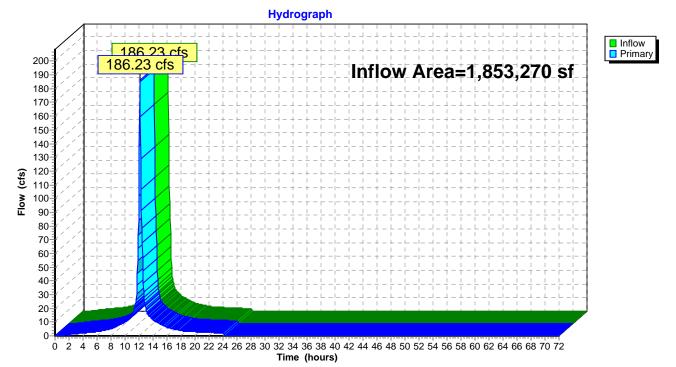


#### Summary for Link 4L: Point of Analysis

Inflow Are	a =	1,853,270 sf,100.00% Impervious, Inflow Depth = 4.93" for 1	0-YR event
Inflow	=	186.23 cfs @ 12.14 hrs, Volume= 761,835 cf	
Primary	=	186.23 cfs @ 12.14 hrs, Volume= 761,835 cf, Atten= 0%,	, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

#### Link 4L: Point of Analysis

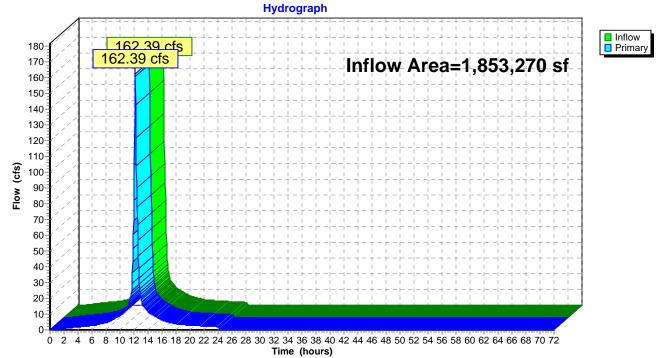


#### Summary for Link 5L: Point of Analysis

Inflow Are	a =	1,853,270 sf, 90.00% Impervious, Inflow Depth = 4.69" for	10-YR event
Inflow	=	162.39 cfs @ 12.18 hrs, Volume= 724,270 cf	
Primary	=	162.39 cfs @ 12.18 hrs, Volume= 724,270 cf, Atten= 0%	o, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## Link 5L: Point of Analysis



**25-YEAR STORM EVENT** 

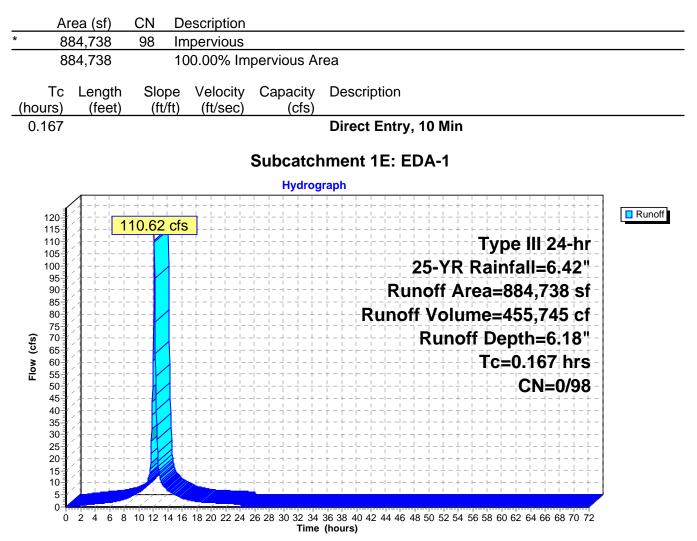
Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1E: EDA-1	Runoff Area=884,738 sf 100.00% Impervious Runoff Depth=6.18" Tc=0.167 hrs CN=0/98 Runoff=110.62 cfs 455,745 cf
Subcatchment 1P: PDA-1	Runoff Area=1,282,652 sf 92.96% Impervious Runoff Depth=6.00" Tc=0.223 hrs CN=74/98 Runoff=142.56 cfs 640,856 cf
Subcatchment 2E: EDA-2	Runoff Area=710,936 sf 100.00% Impervious Runoff Depth=6.18" Tc=0.167 hrs CN=0/98 Runoff=88.89 cfs 366,217 cf
Subcatchment 2P: PDA-2	Runoff Area=570,618 sf 83.34% Impervious Runoff Depth=5.74" Tc=0.223 hrs CN=74/98 Runoff=61.25 cfs 273,022 cf
Subcatchment3E: EDA-3	Runoff Area=257,596 sf 100.00% Impervious Runoff Depth=6.18" Tc=0.167 hrs CN=0/98 Runoff=32.21 cfs 132,693 cf
Link 4L: Point of Analysis	Inflow=231.71 cfs 954,655 cf Primary=231.71 cfs 954,655 cf
Link 5L: Point of Analysis	Inflow=203.82 cfs 913,878 cf Primary=203.82 cfs 913,878 cf
Tatal Dum off Amon 0 700	Edo of Dumoff Valume 4.000 EDO of Automatic Dumoff Double C.00

Total Runoff Area = 3,706,540 sf Runoff Volume = 1,868,533 cf Average Runoff Depth = 6.05" 5.00% Pervious = 185,327 sf 95.00% Impervious = 3,521,213 sf

#### Summary for Subcatchment 1E: EDA-1

Runoff = 110.62 cfs @ 12.14 hrs, Volume= 455,745 cf, Depth= 6.18"



#### Summary for Subcatchment 1P: PDA-1

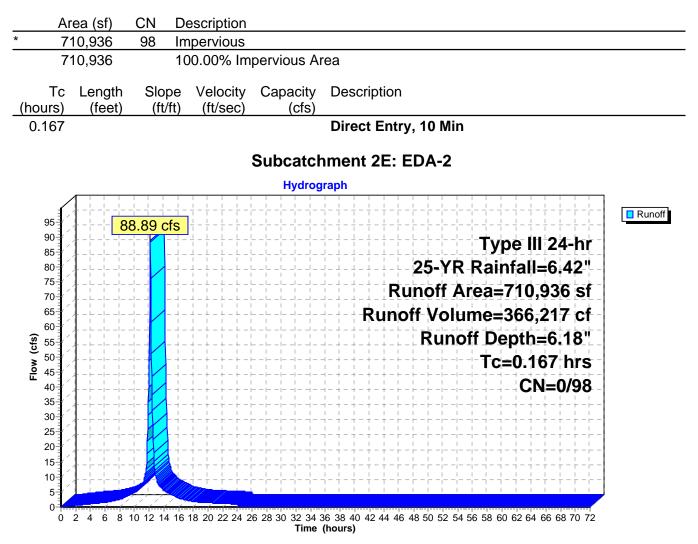
Runoff = 142.56 cfs @ 12.18 hrs, Volume= 640,856 cf, Depth= 6.00"

	Ar	ea (sf)	CN D	escription			
*		92,380		npervious			
*	ę	90,272	74 P	ervious			
		82,652		eighted Av			
		90,272		04% Perv			
	1,19	92,380	92	2.96% Imp	ervious Are	a	
	Тс	Length	Slope	Velocity	Capacity	Description	
(ho	urs)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Description	
	.223			, , , , , , , , , , , , , , , , , , ,		Direct Entry, 13.4	
					Subcatch	nment 1P: PDA-1	
					Hydrog	raph	
	150-		42.56 cfs		-  - + - + - + -		Runoff
	140				-¦¦¦¦¦ 	Type III 24-hr	
	130				+-+-+-	25-YR Rainfall=6.42"	
	120				-'''''' 		
	110		+		-   + - + -	Runoff Area=1,282,652 sf	
	100-				-!!###-             	Runoff Volume=640,856 cf	
(s			<u>-</u>			Runoff Depth=6.00"	
Flow (cfs)	80				-   + -+ -             	Tc=0.223 hrs	
Flov	70		т т т т т т т т т т т т т т т т т т т				
	60				-  +-+-+- 		
	50						
	40				+-+-+-	- + - p - p - p - q - q - q - q - + - + - p - p - p - q - q - q - + - + - p - p - q - q - q - q - q - q - + - + - +	
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	10 <del>-</del>						
	1		1 1 1 1				



#### Summary for Subcatchment 2E: EDA-2

Runoff = 88.89 cfs @ 12.14 hrs, Volume= 366,217 cf, Depth= 6.18"



#### Summary for Subcatchment 2P: PDA-2

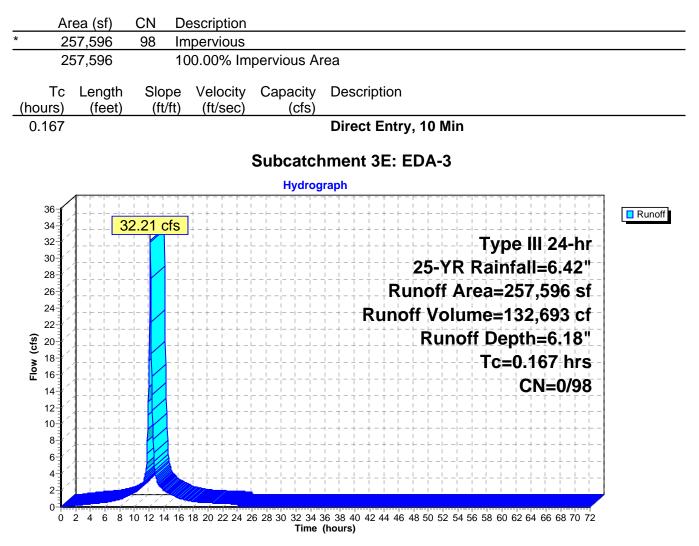
Runoff = 61.25 cfs @ 12.18 hrs, Volume= 273,022 cf, Depth= 5.74"

Area (sf)	CN De	scription			
475,563		pervious			
95,055		rvious			
570,618	94 We	eighted Ave			
95,055 475,563		66% Pervio 34% Imper		3	
475,505	00.	5470 imper		a	
Tc Leng			Capacity	Description	
(hours) (fee	et) (ft/ft)	(ft/sec)	(cfs)		
0.223				Direct Entry, 13.4	
		S	ubcatch	iment 2P: PDA-2	
		Ŭ	Hydrog		
			i i j j		
65	61.25 cfs				Runoff
60				Type III 24-hr	
55				25-YR Rainfall=6.42"	
50			·               	Runoff Area=570,618 sf	
45			·                	Runoff Volume=273,022 cf	
<b>a</b> 40					
( <b>s</b> ) 35-			·       ! ! ! ! !	Runoff Depth=5.74"	
8 30			·¦= =¦= =¦= =¦= = 	Tc=0.223 hrs -	
		$-\frac{1}{1} - \frac{1}{1} - \frac{1}{1} - \frac{1}{1} - \frac{1}{1} - \frac{1}{1}$			
		$-\frac{1}{1}-\frac{1}{1}-\frac{1}{1}-\frac{1}{1}-\frac{1}{1}-\frac{1}{1}$		······································	
				$\begin{array}{cccccccccccccccccccccccccccccccccccc$	
15	-				
		- + - + - +		· · · · · · · · · · · · · · · · · · ·	
5					
0					

#### Summary for Subcatchment 3E: EDA-3

Runoff = 32.21 cfs @ 12.14 hrs, Volume= 132,693 cf, Depth= 6.18"

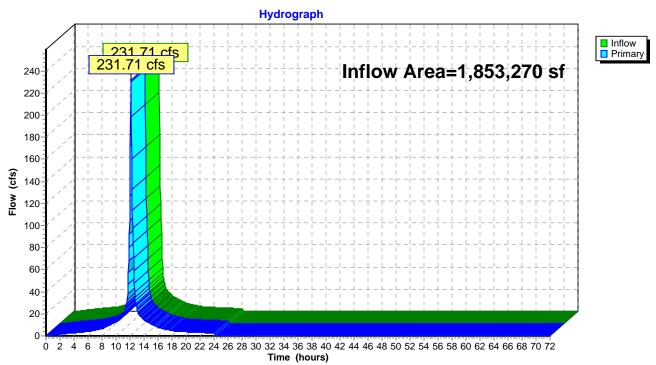
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 25-YR Rainfall=6.42"



### Summary for Link 4L: Point of Analysis

Inflow Are	a =	1,853,270 sf,100.00% Impervious,	Inflow Depth = 6.18" for 25-YR event
Inflow	=	231.71 cfs @ 12.14 hrs, Volume=	954,655 cf
Primary	=	231.71 cfs @ 12.14 hrs, Volume=	954,655 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs



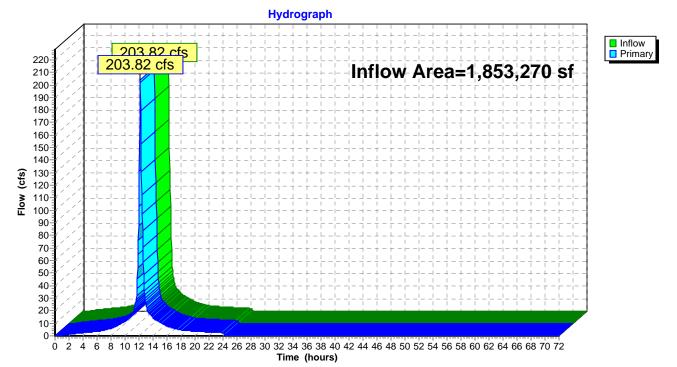
### Link 4L: Point of Analysis

### Summary for Link 5L: Point of Analysis

Inflow Are	a =	1,853,270 sf, 90.00% Impervious, Inflow Depth = 5.92" for 25-YR ev	/ent
Inflow	=	203.82 cfs @ 12.18 hrs, Volume= 913,878 cf	
Primary	=	203.82 cfs @ 12.18 hrs, Volume= 913,878 cf, Atten= 0%, Lag= 0	).0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

### Link 5L: Point of Analysis



**100-YEAR STORM EVENT** 

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv. Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

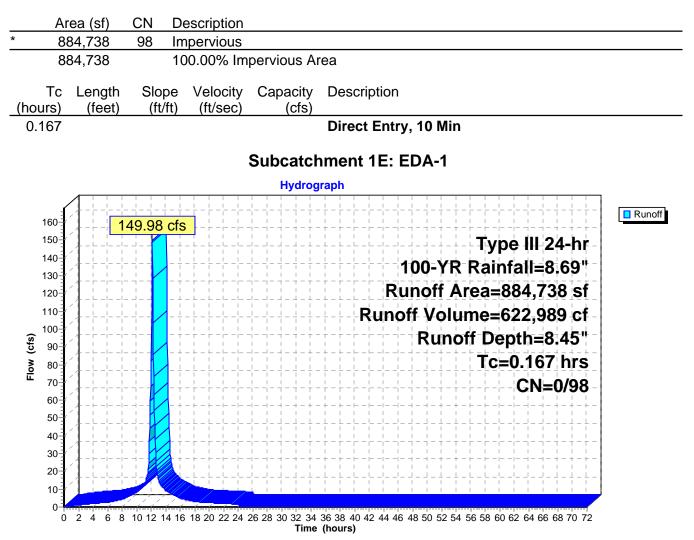
Subcatchment1E: EDA-1	Runoff Area=884,738 sf 100.00% Impervious Runoff Depth=8.45" Tc=0.167 hrs CN=0/98 Runoff=149.98 cfs 622,989 cf
Subcatchment 1P: PDA-1	Runoff Area=1,282,652 sf 92.96% Impervious Runoff Depth=8.25" Tc=0.223 hrs CN=74/98 Runoff=194.70 cfs 881,345 cf
Subcatchment2E: EDA-2	Runoff Area=710,936 sf 100.00% Impervious Runoff Depth=8.45" Tc=0.167 hrs CN=0/98 Runoff=120.52 cfs 500,606 cf
Subcatchment 2P: PDA-2	Runoff Area=570,618 sf 83.34% Impervious Runoff Depth=7.97" Tc=0.223 hrs CN=74/98 Runoff=84.52 cfs 378,808 cf
Subcatchment3E: EDA-3	Runoff Area=257,596 sf 100.00% Impervious Runoff Depth=8.45" Tc=0.167 hrs CN=0/98 Runoff=43.67 cfs 181,386 cf
Link 4L: Point of Analysis	Inflow=314.16 cfs 1,304,981 cf
	Primary=314.16 cfs 1,304,981 cf
Link 5L: Point of Analysis	Inflow=279.22 cfs 1,260,153 cf Primary=279.22 cfs 1,260,153 cf
	of Dunoff Volume - 2 FCF 424 of Average Dunoff Douth - 9 20

Total Runoff Area = 3,706,540 sf Runoff Volume = 2,565,134 cf Average Runoff Depth = 8.30" 5.00% Pervious = 185,327 sf 95.00% Impervious = 3,521,213 sf

#### Summary for Subcatchment 1E: EDA-1

Runoff = 149.98 cfs @ 12.14 hrs, Volume= 622,989 cf, Depth= 8.45"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 100-YR Rainfall=8.69"

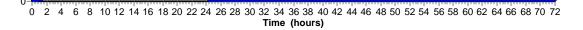


### Summary for Subcatchment 1P: PDA-1

Runoff = 194.70 cfs @ 12.18 hrs, Volume= 881,345 cf, Depth= 8.25"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 100-YR Rainfall=8.69"

	Ar	ea (sf)	CN D	escription			
*		92,380		npervious			
*		90,272	74 P	ervious			
		32,652		eighted Av			
		90,272		04% Pervi			
	1,19	92,380	92	2.96% Imp	ervious Are	ea	
	Тс	Length	Slope	Velocity	Capacity	Description	
_(ho	urs)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
0.	223					Direct Entry, 13.4	
					Subcatch	nment 1P: PDA-1	
					Hydrog		
				 _ L _ L _ L _ L .	· · · · · · ·		
	210		94.70 cfs		-  - + - + - + - -   - + - + - + - + -		Runoff
	200 - 190 -	╱╆╼┝╼┝ <mark>┺┯</mark>			-iiiiiiii	Type III 24-hr	
	180 170					100-YR Rainfall=8.69"	
	160					Runoff Area=1,282,652 sf	
	150 140	,	+		-¦¦¦¦¦ -!!!!	Runoff Volume=881,345 cf	
s)	130 120					Runoff Depth=8.25"	
Flow (cfs)	110				+-+-+-             +-+-	Tc=0.223 hrs	
	100	( <u>}</u>		$-\frac{1}{1}-\frac{1}{1}-\frac{1}{1}-\frac{1}{1}-\frac{1}{1}$			
-	90	/					
	80 <del>-</del> 70 <del>-</del>						
	60						
	50	´ <u></u> } - ¦¦¦		$-\frac{1}{1}$ $-\frac{1}{1}$ $-\frac{1}{1}$ $-\frac{1}{1}$ $-\frac{1}{1}$	$-\frac{1}{1} $	$-\frac{1}{1}-1$	
	40 30-						
	20-				+-+-+-             +-+-		
		/ 1					<i>i</i>

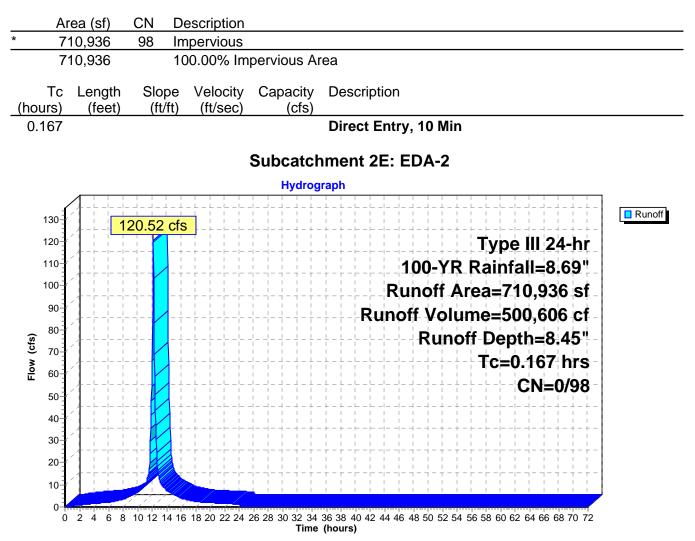


10

#### Summary for Subcatchment 2E: EDA-2

Runoff = 120.52 cfs @ 12.14 hrs, Volume= 500,606 cf, Depth= 8.45"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 100-YR Rainfall=8.69"



### Summary for Subcatchment 2P: PDA-2

Runoff = 84.52 cfs @ 12.18 hrs, Volume= 378,808 cf, Depth= 7.97"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 100-YR Rainfall=8.69"

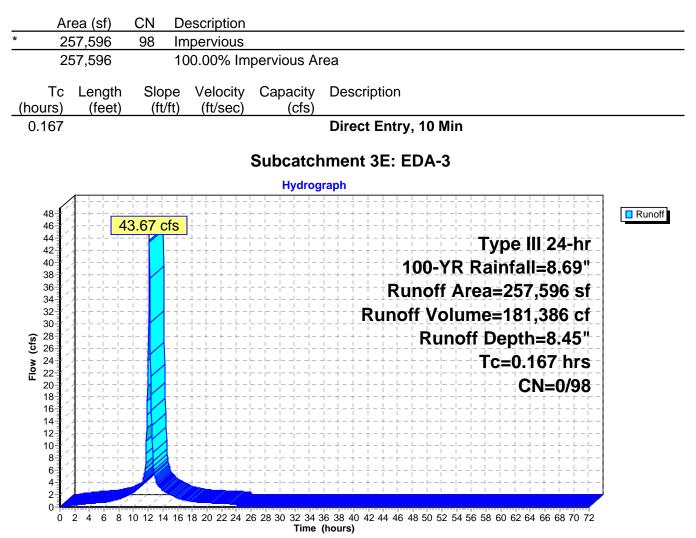
Ar	ea (sf)	CN D	escription			
4	75,563		npervious			
	95,055		ervious			
	70,618		/eighted Av			
	95,055 75,563			vious Area		
4	75,563	0	3.34% imp	ervious Are	a	
Тс	Length	Slope	Velocity	Capacity	Description	
(hours)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
0.223					Direct Entry, 13.4	
				Subcatch	nment 2P: PDA-2	
				Hydrog	jraph	
Í						Runoff
90	<b>8</b>	4.52 cfs				
85 80			- + - + - + - + - + - + - + - + - + - +	 	Type III 24-hr	
80 - 75 -			+ - + - + - + -	- ·	100-YR Rainfall=8.69"	
70			+-+-+-+-	- ·	Runoff Area=570,618 sf	
65			- + - + - + - + -		Runoff Volume=378,808 cf	
60			+ - + - + - + -			
(cts) 50			- + - + - + - + -   	 	Runoff Depth=7.97"	
( <b>clow</b> ( <b>clow</b> )					Tc=0.223 hrs	
- 40-					CN=74/98 -	
35- 30-					$\begin{array}{cccccccccccccccccccccccccccccccccccc$	
25				' ' ' ' ' 		
20						
15	↓			<mark> </mark> <mark> </mark> <mark> </mark> <u> </u> - <u> </u>		
10	<u>}</u>		+-+-+-+-			
5						
	2468	10 12 14 16	18 20 22 24 2	6 28 30 32 34 3	36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72	

Time (hours)

#### Summary for Subcatchment 3E: EDA-3

Runoff = 43.67 cfs @ 12.14 hrs, Volume= 181,386 cf, Depth= 8.45"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr 100-YR Rainfall=8.69"

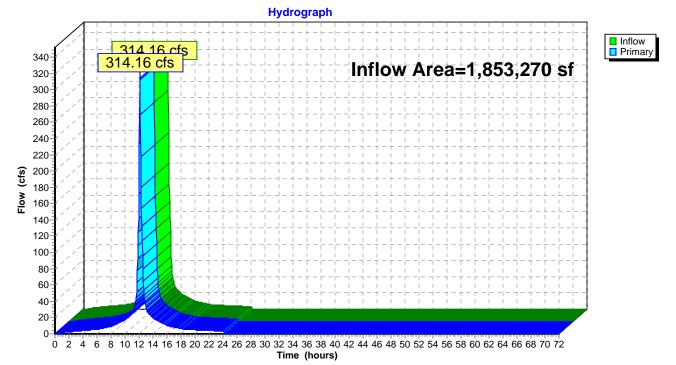


### Summary for Link 4L: Point of Analysis

Inflow Are	a =	1,853,270 sf,	100.00% Impervious,	Inflow Depth = 8.45"	for 100-YR event
Inflow	=	314.16 cfs @	12.14 hrs, Volume=	1,304,981 cf	
Primary	=	314.16 cfs @	12.14 hrs, Volume=	1,304,981 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

### Link 4L: Point of Analysis

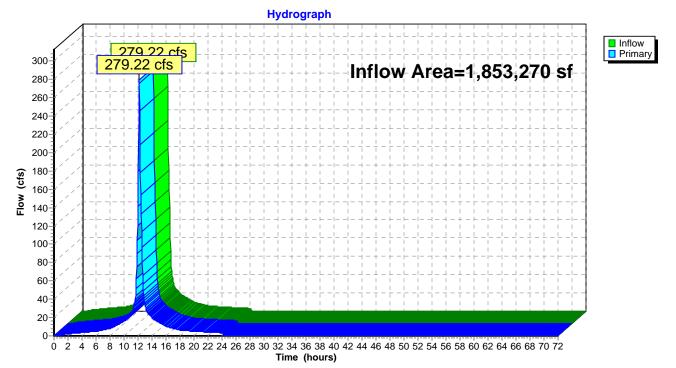


### Summary for Link 5L: Point of Analysis

Inflow Are	a =	1,853,270 sf, 9	90.00% Impervious,	Inflow Depth = 8.16"	for 100-YR event
Inflow	=	279.22 cfs @ 1	2.18 hrs, Volume=	1,260,153 cf	
Primary	=	279.22 cfs @ 1	2.18 hrs, Volume=	1,260,153 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

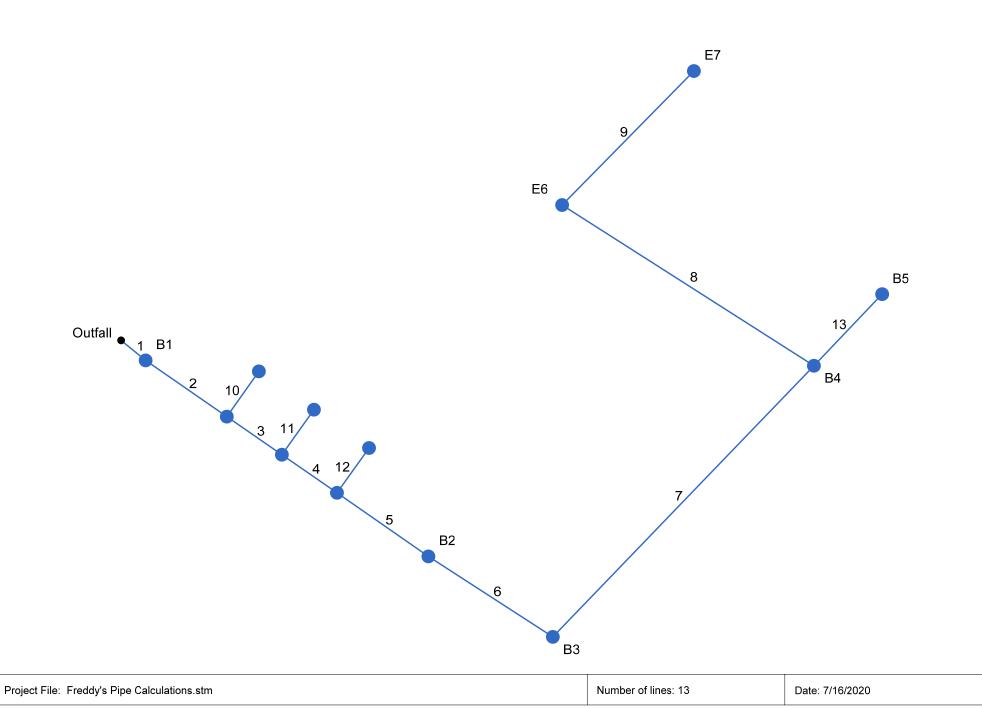
### Link 5L: Point of Analysis



## **B. DESIGN CALCULATIONS**

Pipe Sizing

## Hydraflow Storm Sewers Extension for Autodesk® Civil 3D® Plan

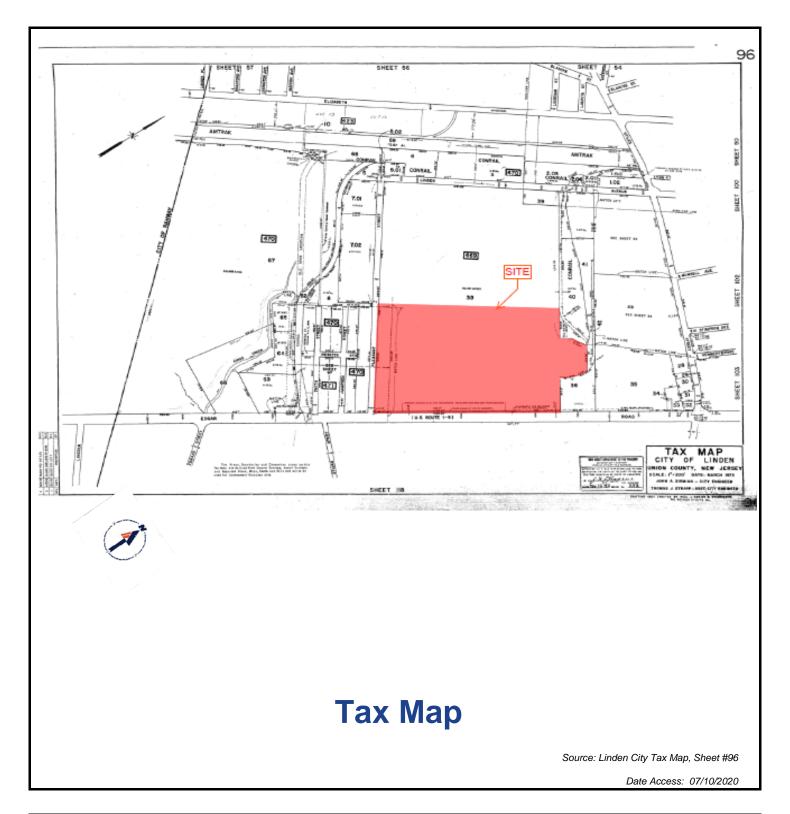


tation	Len	Drng A	rea	Rnoff	Area x	C	Тс			Vel	Pipe		Invert Elev HGL Elev			Grnd / R	Line ID				
ne To		Incr	Total	_coeff	Incr	Total	Inlet	Syst	-(1)	flow	full		Size	Slope	Dn	Up	Dn	Up	Dn	Up	
Line		(ac)	(ac)	(C)			(min)	(min)	(in/hr)	(cfs)	(cfs)	(ft/s)	(in)	(%)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	
End End End End End End End End	10.259 31.971 21.689 36.046 48.000 122.000 96.693 60.885 17.982 17.931 17.880 32.000	0.00 0.00 0.02 0.08 0.11 0.22 0.15 0.02 0.02 0.02	0.76 0.69 0.67 0.65 0.63 0.37 0.15 0.02 0.02 0.02 0.05	0.90 0.00 0.00 0.90 0.87 0.84 0.89 0.90 0.90 0.90 0.79	0.06 0.00 0.00 0.02 0.07 0.09 0.20 0.13 0.02 0.02 0.02 0.04	0.67 0.60 0.58 0.57 0.55 0.53 0.46 0.33 0.13 0.02 0.02 0.02 0.02	0.1 0.1 0.1 0.1 0.1 0.1 0.1 0.1 0.1 0.1	3.0 2.9 2.8 2.7 2.5 2.2 1.6 1.0 0.1 0.1 0.1 0.1 0.1	7.8 7.8 7.8 7.8 7.8 7.8 7.8 7.8 7.8 7.8	5.19 4.70 4.56 4.42 4.28 4.14 3.59 2.57 1.04 0.14 0.14 0.31	5.52 5.71 5.80 5.37 5.62 3.54 3.51 0.27 0.28 3.61	3.54 3.47 3.36 3.35 3.25 3.05 3.14 1.07 2.79 2.79 0.44	x 23 e x 25 x 25	0.29 0.31 0.32 0.28 0.30 0.30 1.00 1.01 0.31	22.87 22.90 23.00 23.07 23.13 23.23 23.38 23.75 24.04 24.50 24.57 24.63 23.75	22.90 23.00 23.07 23.13 23.23 23.38 23.75 24.04 24.22 24.68 24.75 24.81 23.85	23.79 23.98 24.02 24.08 24.10 24.19 24.33 24.54 25.06 24.67 24.74 24.80 24.50	23.79 23.84 23.91 23.92 24.02 24.17 24.50 24.83 25.07 24.89 24.96 25.02 24.50	0.00 27.81 28.25 28.54 28.34 27.56 26.75 26.68 27.40 28.25 28.54 28.34 26.68	27.81 28.25 28.54 28.34 27.56 26.75 26.68 27.40 27.75 28.42 28.42 28.42 26.68	
Project File	· Froddy	- Dia - (	<b>5</b> - 1 1 - 4													r of lines:	10		D	te: 7/16/20	

## **Storm Sewer Tabulation**

### C. MAPS

- Tax Map
- Aerial Map
- Soil Map
- Depth to Water Table Map
- State Planning Area Map
- ♦ USGS Map
- HUC14 Map
- FEMA FIRM Map
- Drainage Area Maps
  - Proposed Drainage Area Map
  - Proposed Inlet Area Map

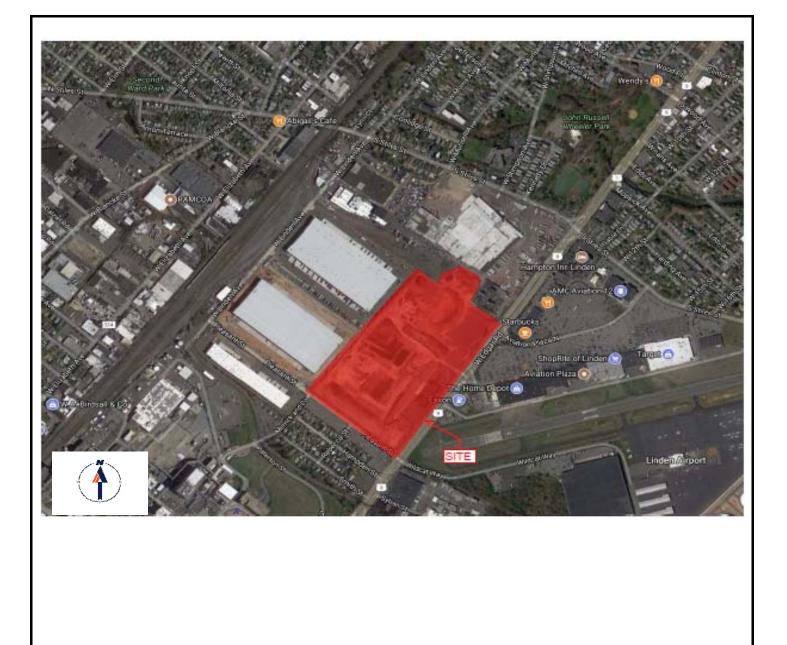


# Linden Development, LLC

810 W. Edgar Road							
Block 469; Lot 38.05							
BENJ #JS200709							
Prepared by: AC	Date: 7/13/2020						
Checked by: JH	Scale: nts						

City of Linden, Union County, NJ

**BOHLER**//



## **Aerial Map**

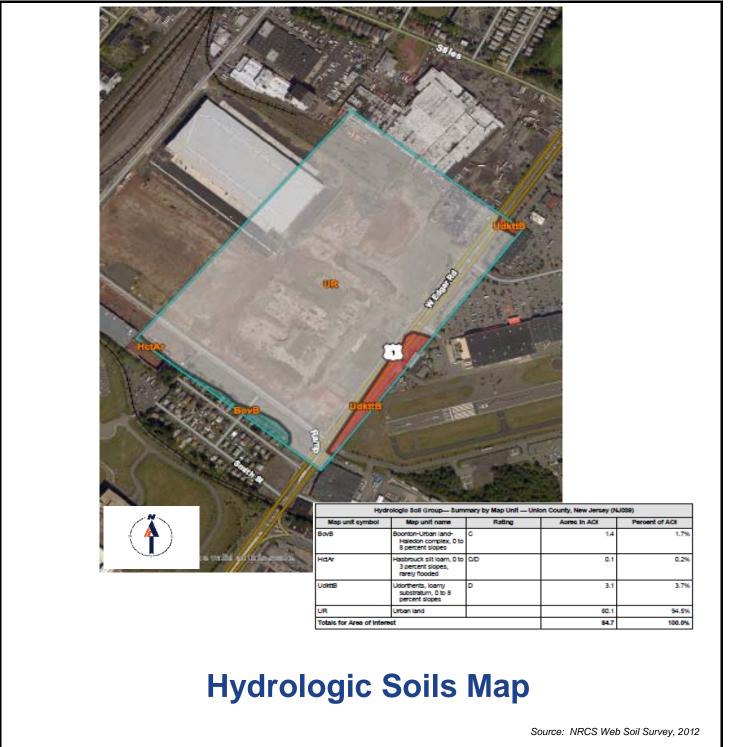
Source: Bing

Date Access: 07/10/2020

## Linden Development, LLC

810 W. Edgar Road Block 469; Lot 38.05							
BEN	IJ #JS200709						
Prepared by: AC	Date: 7/13/2020						
Checked by: JH	Scale: nts						



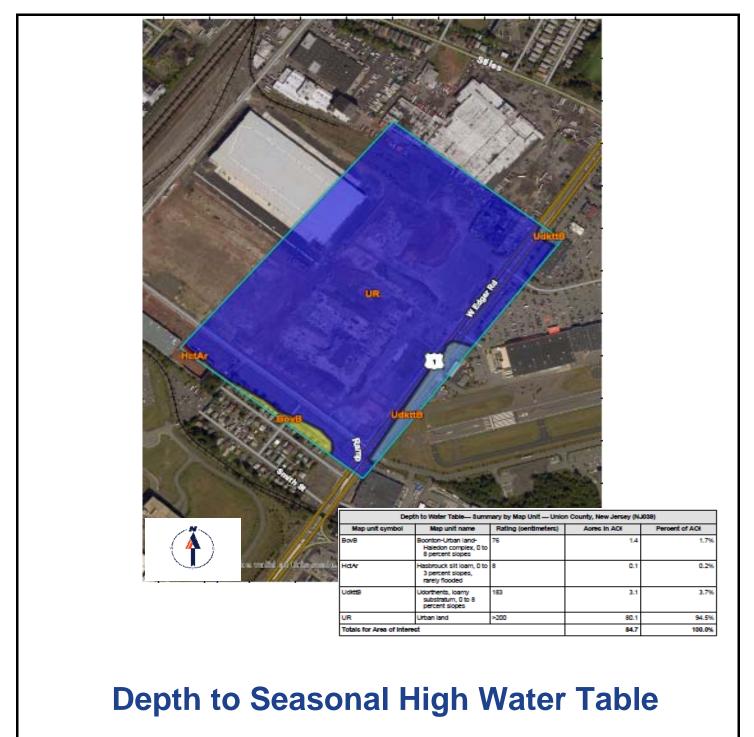


Date Access: 07/10/2020

# Linden Development, LLC

810 W. Edgar Road							
Block 469; Lot 38.05							
BENJ #JS200709							
Prepared by: AC	Date: 7/13/2020						
Checked by: JH	Scale: nts						





Source: NRCS Web Soil Survey, 2012

Date Access: 07/10/2020

## Linden Development, LLC

810 W. Edga. Edgar Road						
Block 469; Lot 38.05						
BENJ #JS200709						
Prepared by: AC	Date: 7/13/2020					
Checked by: JH	Scale: nts					





## **State Planning Area Map**

Source: NJ GeoWeb

Date Access: 07/10/2020

## Linden Development, LLC

810 W. Edgar Road Block 469; Lot 38.05		
		BENJ #JS200709
Prepared by: AC	Date: 7/13/2020	
Checked by: JH	Scale: nts	





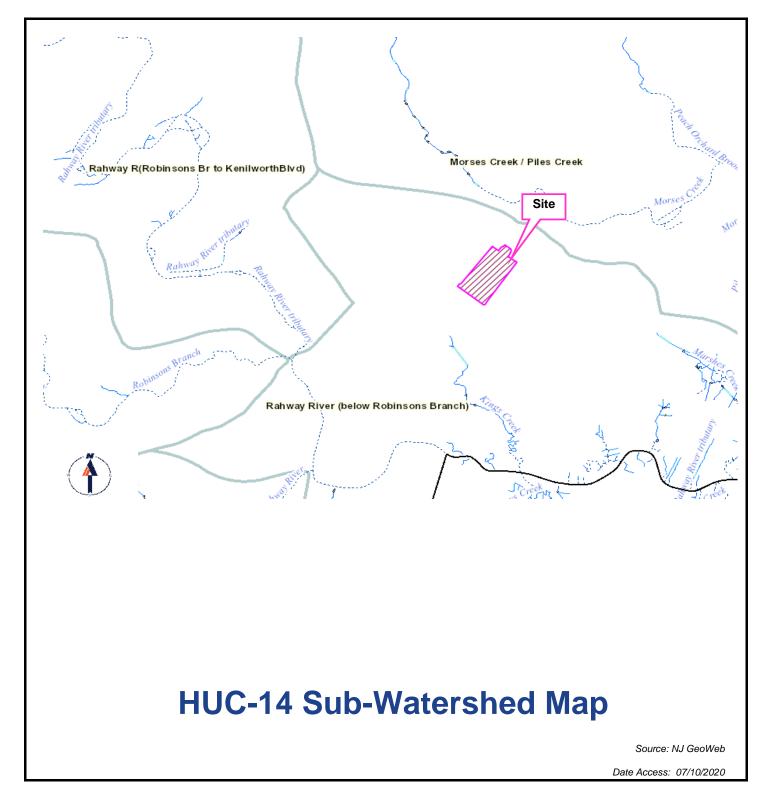
Source: USGS, 1986

Date Access: 07/10/2020

## Linden Development, LLC

810 W. Edgar Road Block 469; Lot 38.05	
Date: 7/13/2020	
Scale: nts	



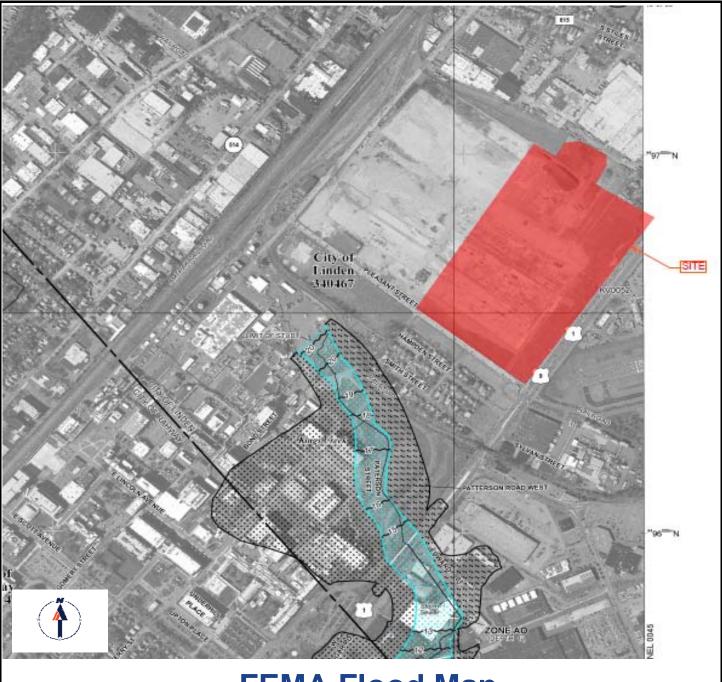


## Linden Development, LLC

810 W. Edgar Road Block 469; Lot 38.05	
BENJ #JS200709	
Prepared by: AC	Date: 7/13/2020
Checked by: JH	Scale: nts

City of Linden, Union County, NJ

**BOHLER**//



## **FEMA Flood Map**

Source:FEMA Preliminary FIRM Map #34039C0044GG, Date 02/03/2015

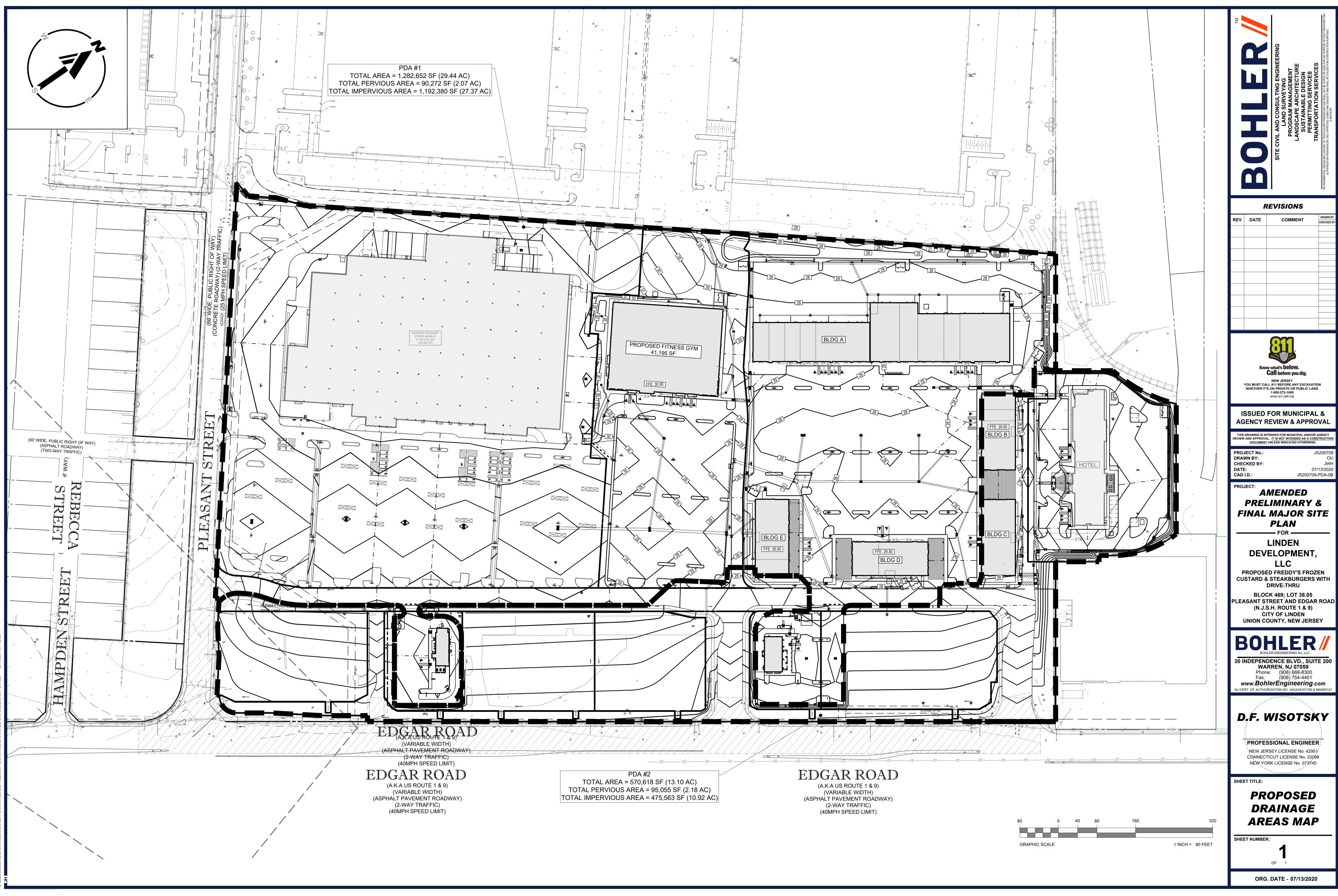
Date Access: 07/10/2020

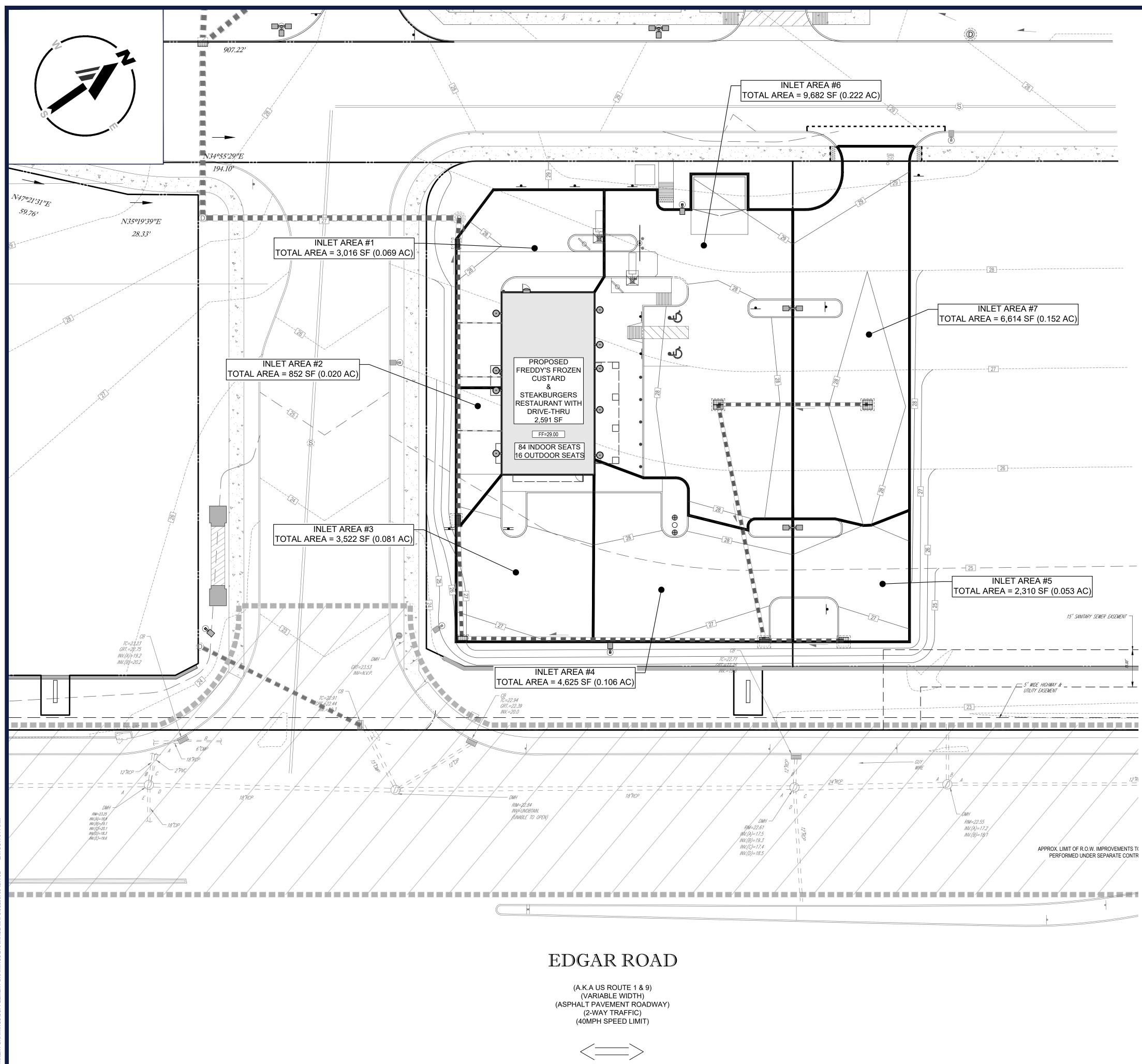
## Linden Development, LLC

810 W. Edgar Road		
Block 469; Lot 38.05		
BENJ #JS200709		
Prepared by: AC	Date: 7/13/2020	
Checked by: JH	Scale: nts	

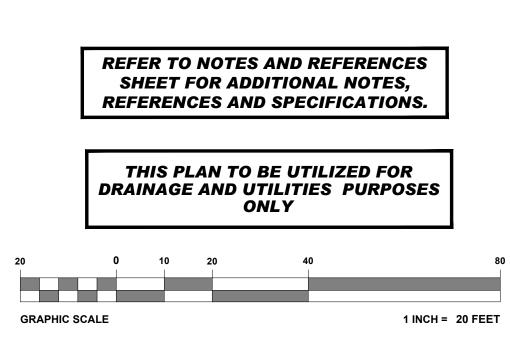
City of Linden, Union County, NJ

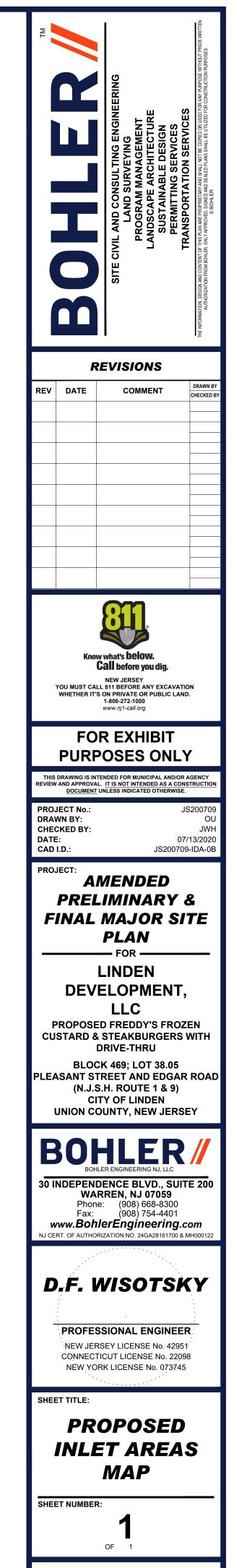
**BOHLER**//





(8)	010102 - 03/12/12)
	PROPERTY LINE
600	EXIST. CONTOUR & ELEVATION
601	PROP. FINISH GRADE CONTOUR & ELEVATION
WWW	
WW	– PROP. WATER – EXIST. GAS
GG	
—_E&TE&TE&T	— EXIST. ELECTRIC/TELEPHONE
———— E&T———— E&T————	
ET&CET&CET&C	, , , ,
ET&C ET&C 	PROP. ELECTRIC/TELEPHONE/CABLE     EXIST. OVERHEAD WIRES
OH OH OH	<ul> <li>PROP. OVERHEAD WIRES</li> </ul>
OR	– EXIST. STORM PIPE –
	PROP. STORM PIPE
ORD	
	_
	— EXIST. SANITARY PIPE —
OR S	PROP. SANITARY PIPE
-~->	PROP. DIRECTION OF DRAINAGE FLOW ARROW
123.45 ×	EXIST. ELEVATION
× TC 123.45	EXIST. TOP OF CURB ELEVATION
× <i>G 122.95</i>	EXIST. GRADE ELEVATION
TW XXX.XX GH XXX.XX GL XXX.XX	PROP. TOP OF WALL ELEVATION PROP. GRADE ON HIGH SIDE OF WALL PROP. GRADE ON LOW SIDE OF WALL
TC XXX.XX G XXX.XX	PROP. TOP OF CURB & FINISHED GRADE ELEV.
	EXIST. AREA/YARD LIGHT
œ	PROP. AREA/YARD LIGHT
C/O	PROP. CLEAN OUT
<b>*</b>	EXIST. INLET
	PROP. INLET
	EXIST. MANHOLE
	PROP. STORM MANHOLE
	PROP. SANITARY MANHOLE
ж.	EXIST. HYDRANT
ж.	PROP. HYDRANT
$\bowtie$	EXIST. UTILITY VALVE
$\bowtie$	PROP. UTILITY VALVE
- <b>e</b>	EXIST. UTILITY POLE PROP. UTILITY POLE
	EXIST. TRAFFIC SIGNAL
	PROP. TRAFFIC SIGNAL





ORG. DATE - 07/13/2020